

Influence of Bed Ash and Fly Ash Replacement in Mortars

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ABSTRACT

The study evaluates the influence of fly ash and bottom ash as partial cement substitutes in mortars by studying the particle size distribution, consistency, flow, fresh density, air content, compressive strength and flexural strength characteristics. The results revealed that fly ash and cement had relatively the same particle size distribution unlike bottom ash. In the fresh state, as the amount of pozzolans increased in the mixtures, the mortars showed an enhancement in workability, were susceptible to water loss by bleeding, and exhibited a decline in fresh density. The early strength gains of the fly ash samples were low but reached higher than the control after 28 days of curing. The flexural strength increased as the fly ash content rose to reach a maximum at 20 % replacement. However, the 2-day compressive strength of bottom ash samples was higher than the control but decreased after 28 days of curing while the flexural strength declined with addition of bottom ash except at 5 % substitution.

Keywords - Fly ash, Bottom ash, Mortars, Compressive strength, Flexural strength

I. INTRODUCTION

Mortar is a mixture of sand, cement or lime and water which is used in the binding of masonry blocks of stone, brick and blocks [1]. The knowledge about the fresh and hardened characteristics of mortar is essential to ensure a good stress distribution in masonry walls. Water to cement ratio and aggregate grading are among several variables that influence physical and mechanical behavior of mortars [2]. Mortar prepared from Ordinary Portland Cement has been a very widespread construction material. However, it has some disadvantages such as late hardening, low tensile strength, large drying shrinkage and low chemical resistance [3]. Many attempts are being made to eliminate these drawbacks, such as the partial replacement of cement by fly ash (FA) and bottom ash (BA) which are residues from the combustion of coal.

The physical properties and the chemicals namely arsenic, mercury, lead, and over a dozen of other heavy metals present in coal ash are responsible for their potential threats [4-6]. Likewise, fine particulate coal ash can damage terrestrial vegetation and aquatic sediments in nearby rivers or lakes [7-9]. The continuous reduction of natural resources and the environmental hazards posed by the disposal of coal ash is reaching an alarming proportion. Hence the use of coal ash as a secondary material will become a necessity rather than a desire. The use of FA and BA (pozzolanic material) in mortars will create a new dimension and if applied on a large scale, shall bring a revolution in the construction industry by economizing on the construction cost and decreasing the amount of coal ash disposed to landfills and

alleviating the environmental hazards caused by the ashes.

Consequently, the aim of this paper is to investigate the effect of FA and BA on the physical properties of mortars as well as to determine the appropriate mortar mix which can utilize efficiently the two waste materials under investigation.

II. MATERIALS AND METHODS

2.1 Overview

The study deals with investigations of two sets of six mixes, one with untreated FA and the other with untreated BA; comparing them against a control mix of Grade 25 N/ mm². Consequently, determining the effects of FA and BA on mortars, 5 %, 10 %, 15 %, 20 %, 25 % and 30 % cement replacement by weight (wt) were attempted.

Table 2.1 below shows the description of the mortar mixes used during this study.

2.2 Materials

2.2.1 Fly Ash and Bottom Ash

The untreated FA and BA used were obtained locally from sub-bituminous coal. Class C FA was used and the BA prior to testing.

2.2.2 Fine aggregate

Local fine basalt aggregates were used with grading of 0/ 2 mm.

2.2.3 Cement and water

Type 1 OPC was used as main binding material. Conversely, tap water was used during the experiment, free from any impurities such as oil,

acid, alkali, salt, organic matter or other hazardous substances.

2.2.4 Mortar Mix Proportions

The mix design was prepared bearing in mind the traditional mortar used by masons, having a cement to aggregate ratio of 1:3 by weight. The amount of cement for each mix was the main variable parameter in the mortar mix. Cement was replaced by FA and BA in increasing percentages from 5 % to 30 % by weight, fixing the amount of fine aggregates used. The sample used was prepared on a 10 kg basis for all the tests.

Table 2.1: Description of mortar mixes

Label	Mortar Description
Control	No replacement of FA or BA with cement/aggregate wt ratio 1:3
FA05	5 % wt replacement of cement by FA
FA10	10 % wt replacement of cement by FA
FA15	15 % wt replacement of cement by FA
FA20	20 % wt replacement of cement by FA
FA25	25 % wt replacement of cement by FA
FA30	30 % wt replacement of cement by FA
BA05	5 % wt replacement of cement by BA
BA10	10 % wt replacement of cement by BA
BA15	15 % wt replacement of cement by BA
BA20	20 % wt replacement of cement by BA
BA25	25 % wt replacement of cement by BA
BA30	30 % wt replacement of cement by BA

2.3 Methods

Tests were performed both on fresh and hardened mortars. Consistency, flow, fresh density and air content tests were done on the fresh mortar while flexural strength and compressive strength tests were done on hardened mortar prisms of dimensions 160 mm × 40 mm × 40 mm. Particle size distribution by sieve analysis (sieves apertures of 8.00 mm, 4.00 mm, 2.00 mm, 1.00 mm, 0.500 mm, 0.250 mm, 0.125 mm and 0.063 mm) was done on all the dry mixed samples. All the tests were performed as illustrated in Table 2.2, based on BS EN 1015: Methods of test for mortar for masonry series.

The fly ash, bottom ash and fine aggregates were all placed in the oven for drying before mixing of the samples. All the ingredients were weighed on a balance with an accuracy of 0.5 grams except the water was measured using a measuring cylinder. All the ingredients were first dry mixed to ensure a heterogeneous mixture, followed by wet mixing. The samples were then moulded as per the defined standard and after 24 hours, these were carefully unmoulded and placed in the curing chamber (water curing for 28 days). The prism samples were then tested on 2, 7 and 28 days, respectively for compressive strength and; 28 days for flexural strength.

Table 2.2: Tests carried out on mortar

Tests	Standard
Particle size distribution	BS EN 1015-1
Consistence by flow table	BS EN 1015-3
Bulk density	BS EN 1015-6
Air content	BS EN 1015-7
Flexural & Compressive strength	BS EN 1015-11
Moulding	EN 196

III. RESULTS AND DISCUSSIONS

3.1 Particle Size Distribution

Figures 3.1(a) and 3.1(b) illustrate the percentage particle size distribution for the different mixes. As the amount of FA was increased in the mixes, the particles became coarser than the control; as fly ash is slightly coarser than cement. Similar observation was noted for BA replacement where BA was coarser in grading than the FA used.

3.2 Flow

Figure 3.2 compares the flow characteristics of mixes containing various percentages of fly ash and bottom ash with the control mix. The flow values of FA and BA varied between 187.7 mm to 193.0 mm and 195.3 mm to 199.3 mm respectively. This is equivalent to an increase of 1.08 % to 3.95 % and 5.21 % to 7.26 % correspondingly in the flow values compared to the control. The use of the ashes increases the net volume of cementitious materials compared to the traditional mortar. Hence, the paste volume is boosted, leading to a reduction in aggregate particle interference and enhancement in mortar workability. However, it can be observed that the flow characteristics of bottom ash mixes are higher than that fly ash mixes as the former lose more water, resulting in higher flow for the same percentage replacement of cement.

3.3 Fresh Density

Figure 3.3 shows the effect of FA and BA replacement on fresh density of mortars. An increase in fly ash and bottom ash caused a general decrease

in fresh density of paste from 2328 to 2265 kg/m³ and 2335 to 2279 kg/m³ respectively. The net decrease can be attributed to the very low specific gravity of fly ash and bottom ash and since the bottom ash is denser than the fly ash, the fresh density of fly ash mixes is lower.

3.4 Air Content

Figure 3.4 shows the air content values in the mortar mixes compared to the control mix. There is a decrease in the values of air content of the fly ash and bottom ash samples compared to the control. The values range between 2.6 % to 3.0 % for FA05 to FA30 and 3.8 % to 4.6 % for BA05 to BA25, while the air content for the control is 4.6 %. This is mainly due to the fact that FA and BA particles are very small and tend to fill the voids in the mortar paste which reduces the air space in the mix. However, it is seen that as the amount of bottom ash exceeds 25 %, the air content becomes higher than the control. It can thus be deduced that at some point the bottom ash stops filling the voids and the air content will not be affected by more addition.

3.5 Consistency

Figure 3.5 shows the effect of fly ash and bottom ash replacement on consistency of mortar paste. The consistency values lie between 16 % to 19 % and 18 % to 20 % in FA and BA replacement respectively. The control mix had a consistency of 18 %. It is also noted that the amount of water needed to get a workable paste at 5-30% replacement is lower than that for the control mix. This is because the fine particles of FA get absorbed on the oppositely charged surfaces of cement particles and prevent them from forming flocs. The cement particles are thus effectively dispersed and trap large amounts of water; implying that the system will have a reduced water requirement to achieve a given consistency. Moreover, the spherical shape and the smooth surface of FA particles help to reduce the inter particle friction and hence, ease mobility. Finally, the "particle packing effect" is also responsible for the reduced water demand [10]. On the other hand, the amount of water needed to get a workable paste for 10-30 % bottom ash replacement is higher than that for the control mix. As the percentage of BA was increased, the amount of cement needed to coat the BA particles decreased, therefore increasing friction between the bottom ash particles. Due to the high water absorption of bottom ash increases, the latter drew more water and cement paste for lubrication between aggregates.

3.6 Compressive Strength

Figure 3.6 illustrates the average compressive strength of FA and BA mixes for 2, 7 and 28 days. Early strength gain, within the first 2 days, decreases as more fly ash is present which is probably due to the free lime reacting during the curing process. The 7 day compressive strength gradually rises with increase in fly ash replacement to reach a maximum strength at 20 % replacement. After 28 days of curing, fly ash replacement at 10-20 % displays a higher strength gain than the control due to the reaction of pozzolans in the hydration process. However, as the fly ash is enhanced to 25 % in the mix, a decrease in compressive strength is noted as an excess of lime inhibits the hydration process.

On the other hand, for the 2 day strength gain, the compressive strength for the 5 – 20 % bottom ash substitution was higher than the control mix but drops as more bottom ash is added to the mortar. 25 % and 30 % replacement yields lower compressive strength than the control. Since the particles of bottom ash is coarser than that of cement, the bonding between them is inefficient when present in high amount. Similar trend is observed for the 7 days curing. However, after 28 days of curing, the compressive strengths for all the mixes except the 5 % replacement are lower than the control mix.

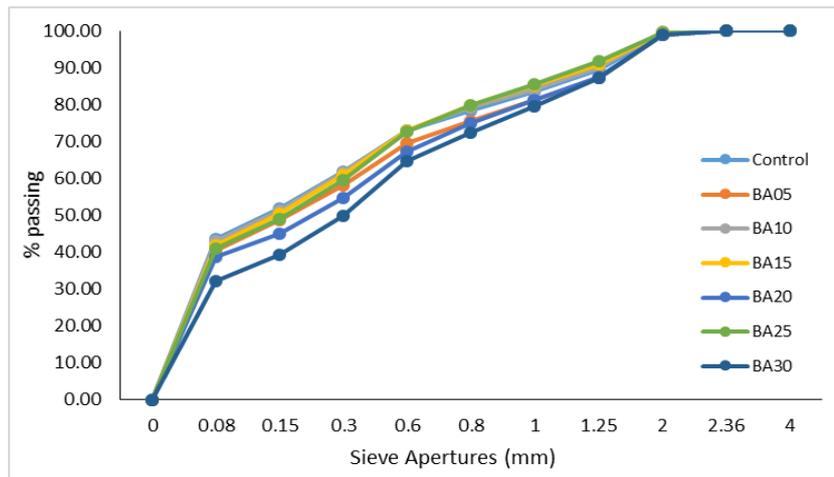


Figure 3.1 (a): Sieve analysis performed on BA mixes

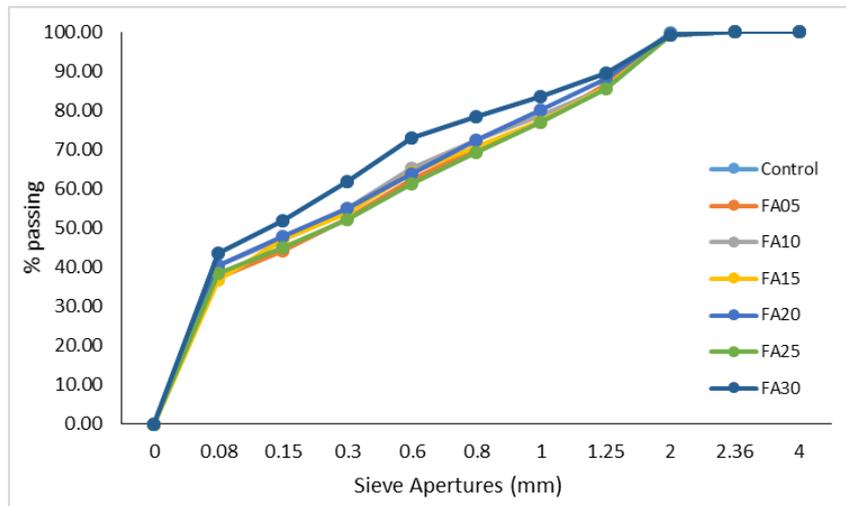


Figure 3.1 (b): Sieve analysis performed on FA mixes

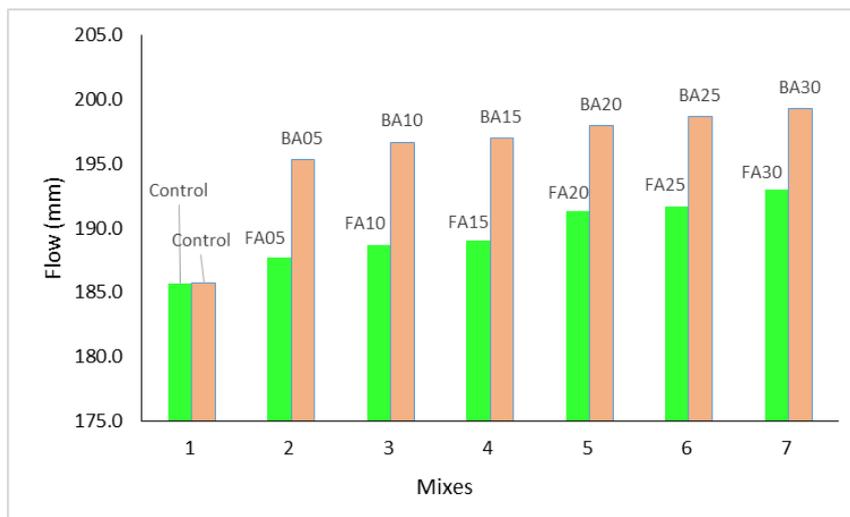


Figure 3.2: Flow characteristics of ash mixes

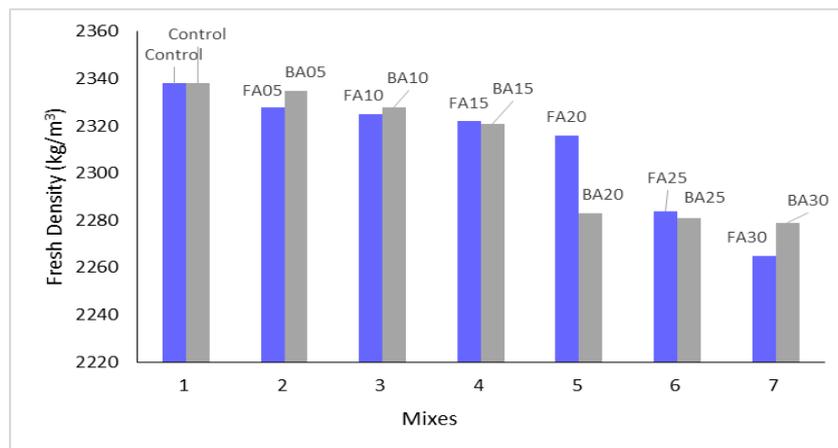


Figure 3.3: Fresh density characteristics of ash mixes

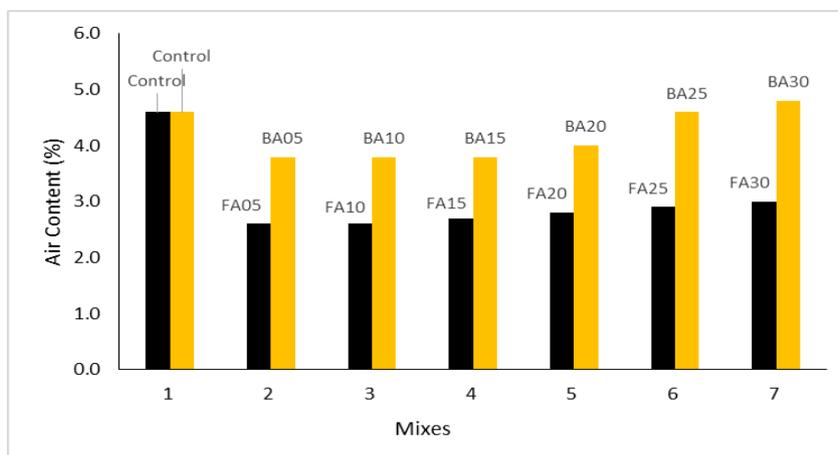


Figure 3.4: Air content of ash mixes

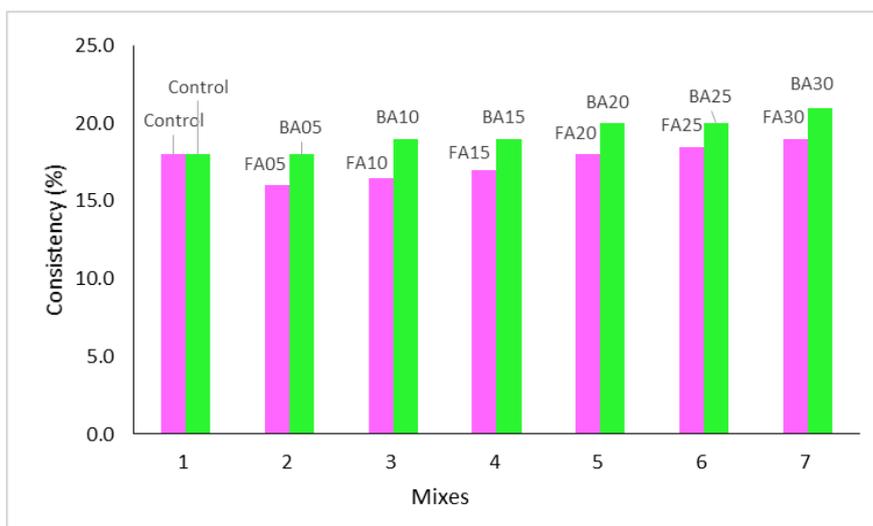


Figure 3.5: Consistency of ash mixes

3.7 Flexural Strength

Figure 3.7 shows the graph of flexural strength variation with % partial cement replacement by ash. The flexural strength for BA is higher than that of FA only at 5% replacement. In all the other mixes, fly ash has higher flexural strength than bottom ash. This is mainly due to the particle size distribution of the different ash. Fly ash particles are finer and enhances the bond strength among the particles and accordingly, its mixes demonstrate higher flexure.

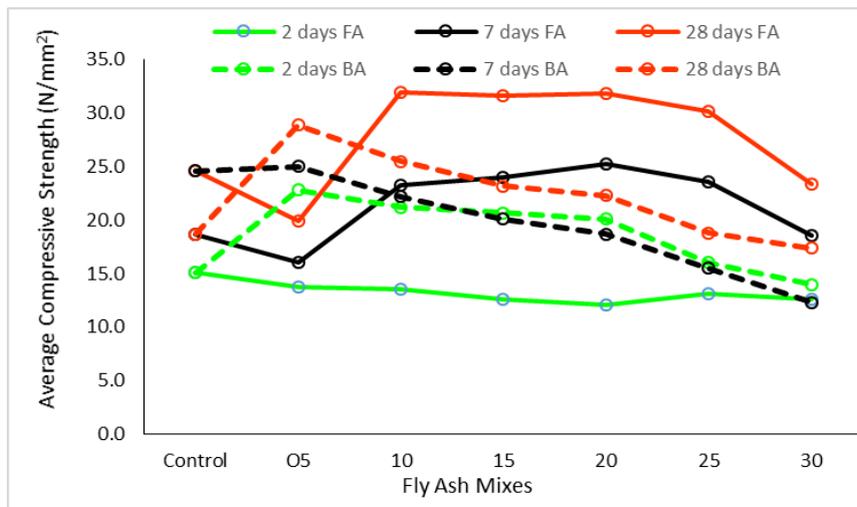


Figure 3.6: Compressive strength of ash mixes

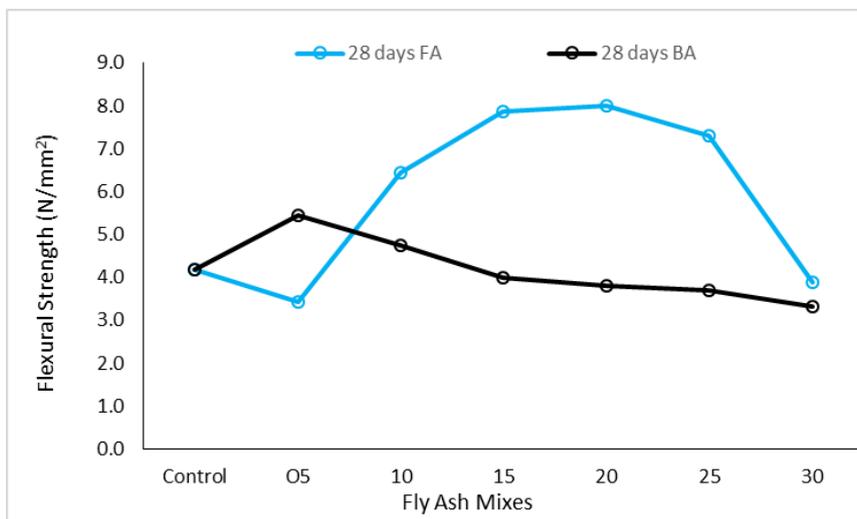


Figure 3.7: Flexural strength of ash mixes

IV. CONCLUSION

This study is based on investigating the use of untreated fly ash and bottom ash in mortars by studying their properties such as particle size distribution, consistency, flow, fresh density, air content, compressive strength and flexural strength characteristics. In the fresh state, as the amount of pozzolans increased in the mixtures, the mortars showed an enhancement in workability, were susceptible to water loss by bleeding, and exhibited a decline in fresh density. The 10-20 % fly ash replacement showed satisfactory results for all the tests and was found to be ideal to be used in mortars. On the other hand, the 5 % bottom ash substitute showed overall satisfactory results and was optimum, according to this research.

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